

Ref: 9026

November 1, 2022

Ms. Valerie Oorthuys Planning Director Town of Stow 380 Great Road Stow, MA 01775

Re: Response to Transportation Peer Review

Proposed Residential Development - Athens Street

Stow, Massachusetts

Dear Ms. Oorthuys:

Vanasse & Associates, Inc. (VAI) is providing responses to the comments that were raised in the October 18, 2022 Transportation Peer Review letter prepared by Green International Affiliates, Inc. (Green) concerning their review of the April 19, 2022 Transportation Impact Assessment (the "April 2022 TIA") that was prepared by VAI in support of the proposed residential community to be located off Athens Street in Stow, Massachusetts (hereafter referred to as the "Project"). Listed below are the comments that were identified by Green in the subject letter followed by our response on behalf of the Project proponent.

Comment 1: We consider the study area to be adequate, complying with the MassDOT 2014

Transportation Impact Assessment (TIA) Guidelines.

Response: No response required

Comment 2: No seasonal adjustment was made to existing traffic volumes as the data were collected in

> June, which was found to experience above-average traffic volumes. To account for any impacts on traffic volumes due to the COVID-19 pandemic, traffic volumes from a nearby permanent count station were compared for June 2019 and June 2021. June 2021 traffic volumes were found to be lower than June 2019 traffic volumes. The June 2021 traffic volumes collected for this project were adjusted upward by the difference between the COVID-19 adjustment factor (10.5 percent) and the seasonal adjustment factor (6.9 percent). We concur with this methodology and no further information is required.

Response: No response required.

Comment 3: No crashes were reported to have occurred at either the Hudson Road and Athens Street

> intersection nor the Hudson Road and Edson Road intersection within the five-year review period. 40 total crashes were reported at the intersection of Hudson Road and Route 117, resulting in a crash rate above the MassDOT statewide and districtwide averages for unsignalized intersections. As design of a traffic signal at this intersection is currently

underway, no further information is required.

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Response: No response required.

Comment 4: No-Build conditions were established by projecting existing traffic volumes to Year 2029,

> a seven-year planning horizon from the date of the TIA. A conservative 1.0 percent compounded annual growth rate was used. Traffic volumes expected to be generated by nearby proposed developments were also added. We concur with this methodology and no

further information is required.

Response: No response required.

Comment 5: Project-generated traffic was estimate using the ITE Trip Generation Manual methodology

for Land Use Code (LUC) 210, Single-Family Detached Housing, and LUC 251, Senior Adult Housing – Single-Family. Because the community will not have an age restriction but will be marketed towards active adults, it was assumed that 50 percent of residential units would be traditional single-family homes (LUC 210) and the remaining 50 percent

would be senior housing (LUC 251).

The description for LUC 251 state that "the development has a specific age restriction for its residents" and that these developments may include amenities such as golf courses and swimming pools. It also advises that "caution should be used when applying trip rates for this land use as it may contain a wide variety of studies ranging from communities with very active, working residents to communities with older, retired residents." The data collected for this land use also includes attached housing such as duplexes and townhouses, which generally result in fewer trips than detached housing. It is also noted that LUC 210, Single-Family Detached Housing accounts for a mix of family types that includes seniors as well as younger adults with/without children. Finally, LUC 251 rates were calculated based on only 15 studies at 6 sites, so the range in the data types will have a greater impact on the results. LUC 210 rates were calculated based on 174 studies and is more accurate for this specific development type.

As this community will not have an age restriction, does not include shared amenities or attached dwelling units, and due to ITE's caution, the Applicant should use LUC 210, Single-Family Detached Housing, for all 141 proposed units.

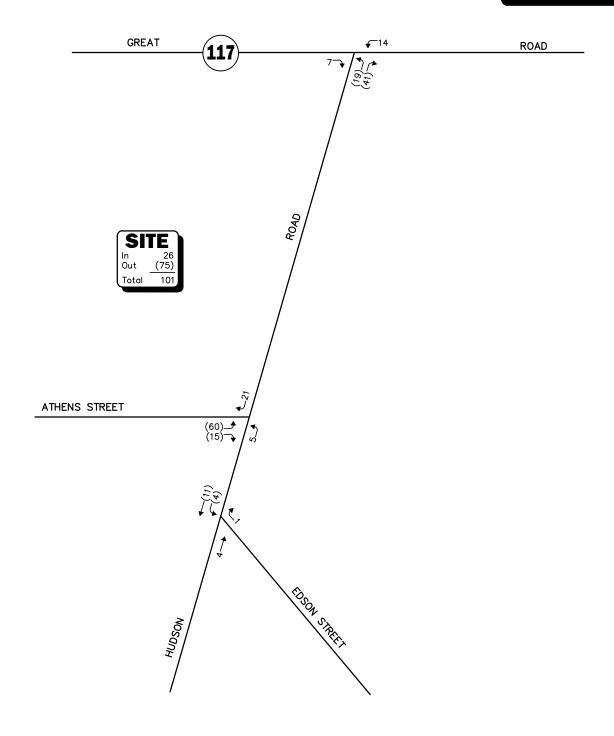
Response:

The trip-generation calculations for the Project have been revised using ITE LUC 210, Single-Family (Detached) Housing, the results of which are summarized in Table 3R and reflect a slight reduction in the number of proposed housing units from 141 to 140. Figures 7R and 8R depict the revised weekday morning and evening peak-hour Projectgenerated trip assignment, with Figures 9R and 10R depicting the revised 2029 Build weekday morning and evening peak-hour traffic volumes.



Legend:

XX Entering Trips (XX) Exiting Trips





Project-Generated Weekday Morning Peak-Hour Traffic Volumes

Figure 7R

Legend:

XX Entering Trips (XX) Exiting Trips

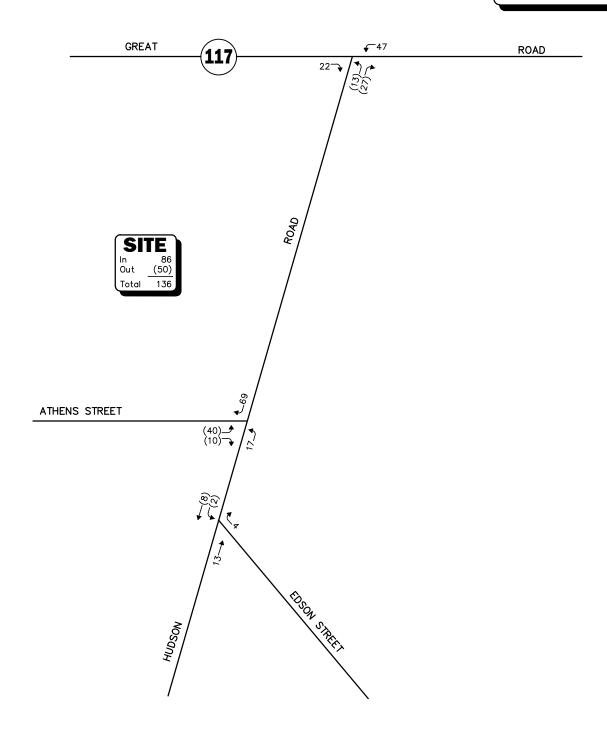




Figure 8R

Project-Generated

Weekday Evening

Peak-Hour Traffic Volumes



Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure 9R



2028 Build Weekday Morning Peak-Hour Traffic Volumes



Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure 10R



2028 Build Weekday Evening Peak-Hour Traffic Volumes

Table 3R TRIP-GENERATION SUMMARY

Time Period/Direction	Single-Family Housing (140 Dwellings) ^a
Average Weekday Daily: Entering Exiting Total	688 688 1,376
Weekday Morning Peak Hour: Entering Exiting Total	26 <u>75</u> 101
Weekday Evening Peak Hour: Entering Exiting Total	86 50 136

^aBased on ITE LUC 210, Single-Family Detached Housing.

Comment 6:

Trip distribution for project-generated traffic was determined using U.S. Census Journey-to-Work data for the Town of Stow, which is standard practice for residential developments. The Applicant should clarify during what year the Census data was collected.

Response:

The U.S. Census Journey-to-Work data was obtained from the 2011-2015 American Community Survey.

Comment 7:

A Level of Service (LOS) analysis was conducted for the three study intersections for Existing, No-Build, and Build conditions. The analysis showed negligible differences between the three scenarios for the intersections of Hudson Road with Athens Street and Edson Street. The Hudson Road and Route 117 intersection is expected to experience LOS F during both 2029 No-Build and 2029 Build scenarios. As previously noted, design of a traffic signal at this intersection is currently underway. Traffic signal timing and phasing should take into account trips expected to be generated by this proposed development.

Response:

The 2029 Build condition traffic operations analysis has been revised to reflect the revised trip-generation calculations for the Project as reflected in Table 3R and shown on Figures 9 and 10. Table 4R summarizes the results of the revised analysis for the 2029 Build condition.

A review of Table 4R indicates no significant changes from the results that were reported in Table 4 of the April 2022 TIA. The traffic volume increase on the Hudson Road approach to Route 117 was shown to result in an increase in vehicle queuing over the conditions that were reported in Table 4; however, with the planned installation of a traffic



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> control signal at the intersection, actual operating conditions at the intersection will be improved from the conditions reported in these tables.

> We concur with Green that the design of the traffic signal timing and phasing at the improved, signalized intersection of Route 117 at Hudson Road should reflect the traffic volumes that are expected to be associated with the Project.

Comment 8:

Stopping sight distances (SSD) and intersection sight distances (ISD) were measured at the intersection of Athens Street and Hudson Road. Minimum required sight distances were met in each direction for both stopping sight distance and intersection sight distance. During our site visit, Green confirmed that the minimum sight distances are available; however, vegetation on both corners has the potential to limit sight distance. Please provide a figure showing site triangles that the Applicant will commit to keeping clear of vegetation.

Response:

A sight triangle plan has been prepared for the intersection of Athens Street/Hudson Road intersection and is attached.

Comment 9:

The TIA makes several recommendations for project access, including a minimum width of 22 feet for internal roadways and placing vehicles exiting Athens Street under STOP-sign control. We concur with all these recommendations. During our site visit, Green observed that Athens Street is significantly sloped at its intersection with Hudson Road, which could cause difficulty for drivers exiting Athens Street. Please confirm whether the Applicant plans to regrade Athens Street in this area to reduce the severity of this slope. Additionally, please confirm whether the required ISD should be re-evaluated per Table 9-5 of AASHTO's "A Policy on Geometric Design of Highways and Streets, 7th Edition" based on the slope of the Athens Street approach.

Response:

The Athens Street approach to Hudson Road will be regraded such the slope will not exceed 3 percent within 50 feet of Hudson Road. The sight distance adjustments for approach grade that are shown in Table 9-5 apply to uncontrolled intersections and would not apply to the Project site driveway which will be under STOP-sign control.

Comment 10: The Applicant will contribute a "fair-share" cost contribution proportionate to the incremental increase in traffic from the project for pedestrian and bicycle improvements along Hudson Road. Please continue to coordinate with the Town regarding this contribution.

Response:

The Project proponent will continue to coordinate with the Town to establish the "fairshare" cost allocation for pedestrian and bicycle improvements along Hudson Road.

Comment 11: The Applicant has contracted with Toole Design Group to complete a Road Safety Audit at the intersection of Hudson Road and Route 117. We find this acceptable and no further information is required.

Response:

No response required. We note that the final Road Safety Audit for the Route 117/ Hudson Road intersection has been published and is available on MassDOT's website (https://gis.massdot.state.ma.us/roadsafetyaudits/)



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Comment 12: The Applicant will install a STOP sign and STOP line on the Edson Street approach to

Hudson Road. Please provide a conceptual layout of the locations of the STOP sign and

STOP line for review.

Response: A conceptual improvement plan depicting the recommended STOP-sign and STOP-line

placement on the Edson Street approach to Hudson Road is attached.

Comment 13: The Applicant commits to improving the intersection of Hudson Road with Walcott Street

and Randall Road by upgrading the existing STOP sign and STOP line, trimming vegetation within the sight triangle areas, and installing "Intersection Ahead" warning

signs. Please provide a figure showing the sight triangles that will be cleared.

Response: A conceptual improvement plan depicting the recommended sign, pavement marking and

sight line improvements at the Hudson Road/Walcott Street/Randall Road intersection is

attached.

Comment 14: The TIA outlines several transportation demand management (TDM) measures aimed at encouraging the use of alternative modes of transportation to single-occupant vehicles.

We concur with the proposed TDM strategies, although we note that the likelihood of transit use is low due to the location of the project site greater than five miles from the

nearest commuter rail station.

Please clarify whether the secure bicycle parking will also be covered to protect bicycles

from inclement weather. Additionally, please clarify whether electric vehicle charging

stations will be provided within the community.

Response: The residential units will have garages that will provide secure, weather protected bicycle

storage. A bicycle rack(s) will be provided at the amenity/community building. The Project proponent will evaluate the installation of electric vehicle (EV) charging accommodations at the amenity/community building. Residents will have the option of

installing EV charging units within their garages.

We trust that this information is responsive to the comments that were raised in Green's October 18, 2022 *Transportation Peer Review* letter. If you should have any questions or would like to discuss our responses in more detail, please feel free to contact me.

Sincerely,

VANASSE & ASSOCIATES, INC.

Leffrey S. Dirk, P.E., PTOE, FITE

Managing Partner

Professional Engineer in CT, MA, ME, NH, RI and VA

Grey S. Dirk

JSD/jsd

Attachments



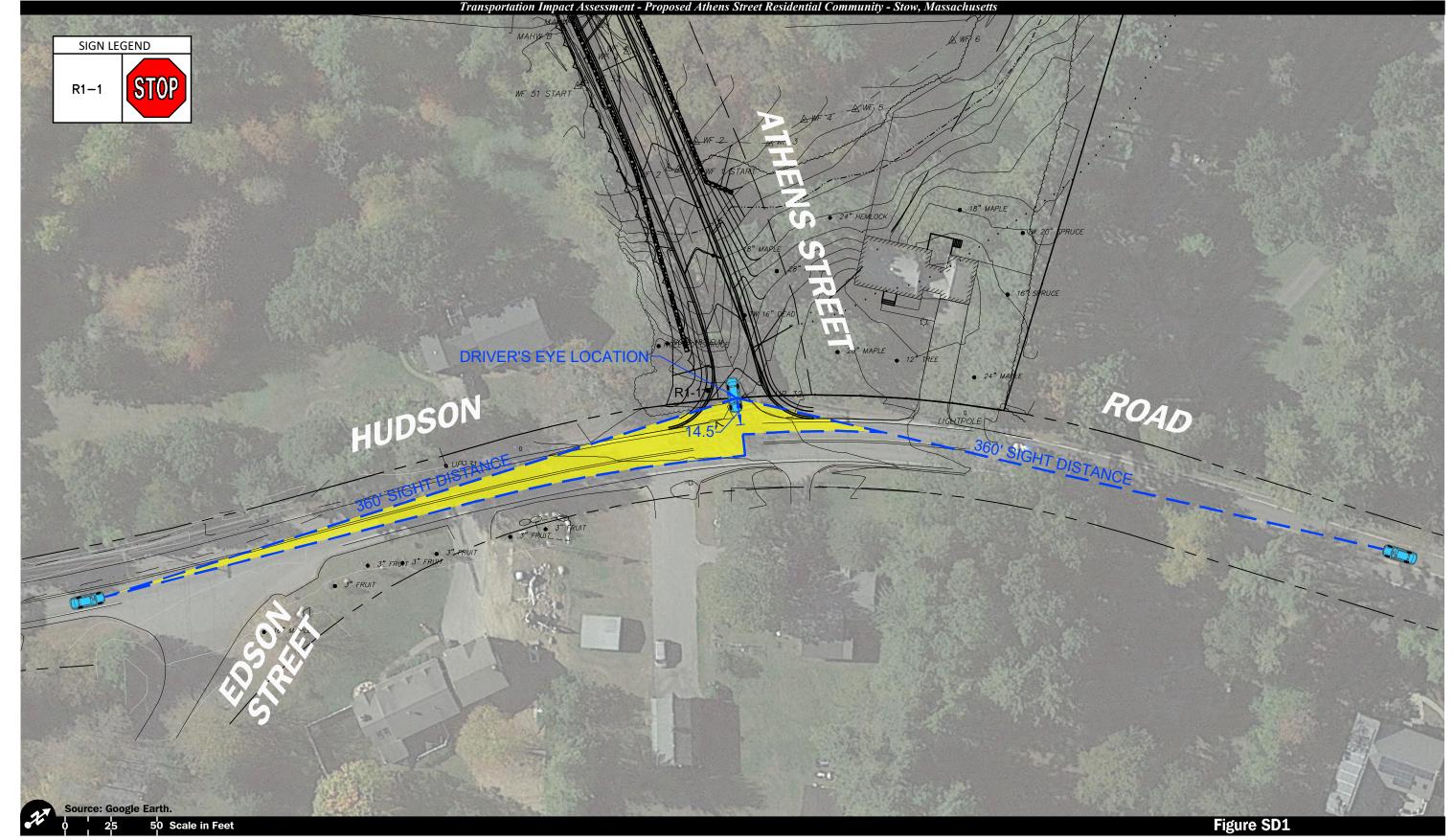
Table 4R UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2021 Ex	isting			2029 No	-Build			2029 E	Build	
Unsignalized Intersection/Peak Hour/Movement	Demanda	Delay ^b	LOSc	Queue ^d 95 th	Demand	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Route 117 at Hudson Road												
Weekday Morning:												
Route 117 EB: TH/RT	528	0.0	A	0	576	0.0	A	0	583	0.0	A	0
Route 117 WB: LT/TH	331	2.5	A	1	368	2.7	A	1	382	3.0	A	1
Hudson Road NB: LT/RT	239	26.3	D	4	302	>50.0	F	9	363	>50.0	F	16
Weekday Evening:												
Route 117 EB: TH/RT	369	0.0	A	0	415	0.0	A	0	437	0.0	A	0
Route 117 WB: LT/TH	784	2.9	A	1	883	3.4	A	1	930	3.8	A	2
Hudson Road NB: LT/RT	202	>50.0	F	7	248	>50.0	F	17	288	>50.0	F	26
Hudson Road at Athens Street Weekday Morning:												
Athens Street EB: LT/RT	0	0.0	A	0	0	0.0	A	0	75	12.3	В	1
Hudson Road NB: LT/TH	222	0.0	A	0	286	0.0	A	0	291	0.1	A	0
Hudson Road SB: TH/RT	126	0.0	A	0	151	0.0	A	0	172	0.0	A	0
Weekday Evening:	120	0.0	А	U	131	0.0	A	U	1/2	0.0	A	U
Athens Street EB: LT/RT	0	0.0	Α	0	0	0.0	Α	0	50	16.6	С	1
Hudson Road NB: LT/TH	193	0.0	A	0	239	0.0	A	0	256	0.5	A	0
Hudson Road SB: TH/RT	295	0.0	A	0	370	0.0	A	0	439	0.0	A	0
Hudson Road at Edson Street Weekday Morning:												
Edson Street WB: LT/RT	21	9.7	A	0	83	10.8	В	1	84	10.9	В	1
Hudson Road NB: TH/RT	204	0.0	A	0	227	0.0	Α	0	231	0.0	Α	0
Hudson Road SB: LT/TH	126	1.0	A	0	151	1.7	A	0	166	1.7	A	0
Weekday Evening:												
Edson Street WB: LT/RT	38	10.1	В	0	81	11.7	В	1	85	12.0	В	1
Hudson Road NB: TH/RT	161	0.0	A	0	191	0.0	A	0	204	0.0	A	0
Hudson Road SB: LT/TH	295	0.5	A	0	370	1.5	A	0	380	1.5	Α	0

NB = northbound, EB = eastbound; SB = southbound; WB = westbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

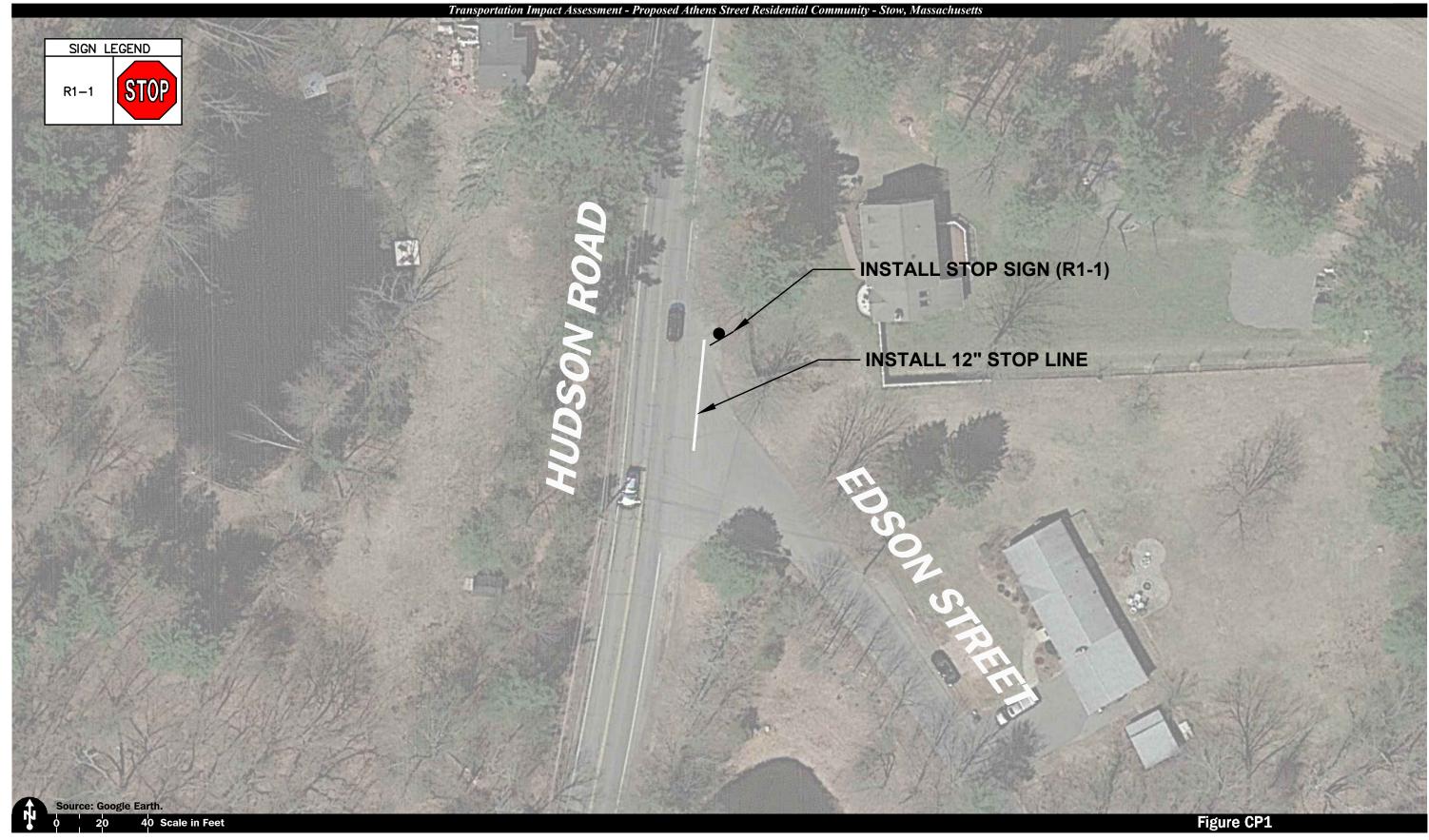
^aDemand in vehicles per hour.
^bAverage control delay per vehicle (in seconds).
^cLevel of service.

^dQueue length in vehicles.





Sight Distance Plan Hudson Road at Athens Street





Conceptual Improvement Plan Hudson Road at Edson Street



Conceptual Improvement Plan Hudson Road at Randall Road and Walcott Street

APPENDIX

TRIP-GENERATION CALCULATIONS CAPACITY ANALYSIS WORKSHEETS





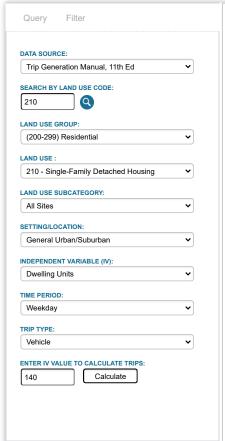


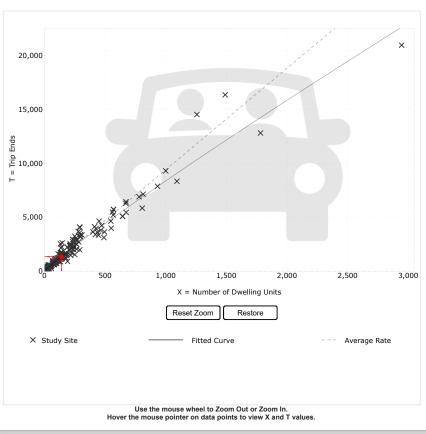


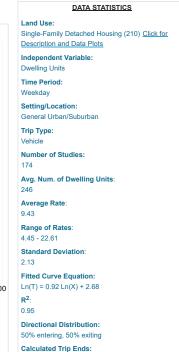
Graph Look Up

Data Plot and Equation

ITETripGen Web-based App Graph Look Up TGM Desk Reference Add Users Comments







Average Rate: 1320 (Total), 660 (Entry), 660 (Exit)

Fitted Curve: 1375 (Total), 687 (Entry), 688 (Exit)



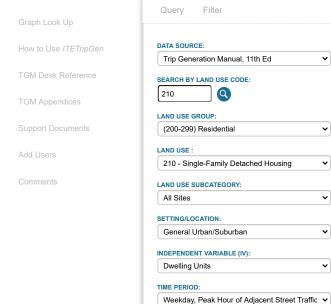






Graph Look Up

ITETripGen Web-based App Query Filter



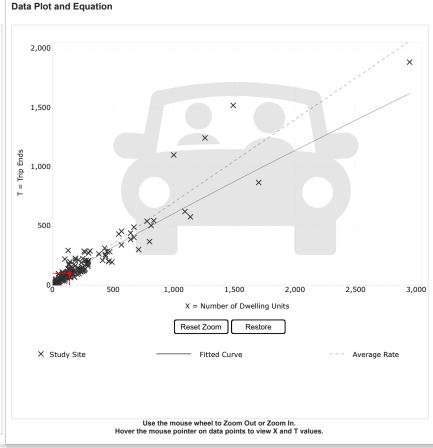
TRIP TYPE:

Vehicle

140

ENTER IV VALUE TO CALCULATE TRIPS:

Calculate





Land Use:

Single-Family Detached Housing (210) Click for Description and Data Plots

Independent Variable:

Dwelling Units

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic

One Hour Between 7 and 9 a.m.

Setting/Location:

General Urban/Suburban

Trip Type:

Vehicle

Number of Studies:

192

Avg. Num. of Dwelling Units: 226

Average Rate:

0.70

Range of Rates:

0.27 - 2.27

Standard Deviation:

0.24

Fitted Curve Equation:

Ln(T) = 0.91 Ln(X) + 0.12

0.90

Directional Distribution:

26% entering, 74% exiting

Calculated Trip Ends:

Average Rate: 98 (Total), 25 (Entry), 73 (Exit) Fitted Curve: 101 (Total), 26 (Entry), 75 (Exit)



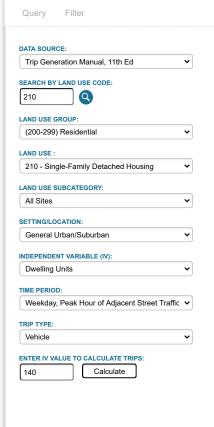






Graph Look Up

ITETripGen Web-based App Graph Look Up TGM Desk Reference Add Users Comments



2,500 2,000 Trip Ends 1,500 1,000 3,000 1,000 1,500 2,000 2,500 X = Number of Dwelling Units Reset Zoom Restore X Study Site Fitted Curve --- Average Rate Use the mouse wheel to Zoom Out or Zoom In. Hover the mouse pointer on data points to view X and T values.

Data Plot and Equation



Land Use:

Single-Family Detached Housing (210) Click for Description and Data Plots

Independent Variable:

Dwelling Units

Time Period:

Weekday

Peak Hour of Adjacent Street Traffic

One Hour Between 4 and 6 p.m.

Setting/Location:

General Urban/Suburban

Trip Type:

Vehicle

Number of Studies:

Avg. Num. of Dwelling Units:

248

Average Rate:

0.94

Range of Rates:

0.35 - 2.98

Standard Deviation:

0.31

Fitted Curve Equation:

Ln(T) = 0.94 Ln(X) + 0.27

0.92

Directional Distribution:

63% entering, 37% exiting

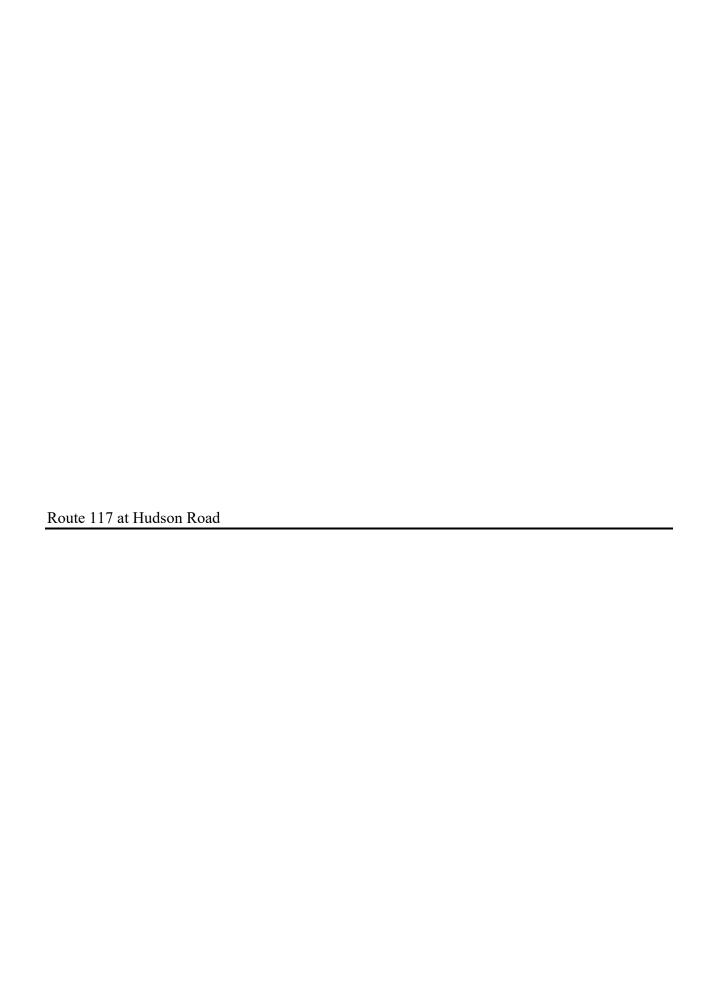
Calculated Trip Ends:

Average Rate: 132 (Total), 83 (Entry), 49 (Exit) Fitted Curve: 136 (Total), 86 (Entry), 50 (Exit)



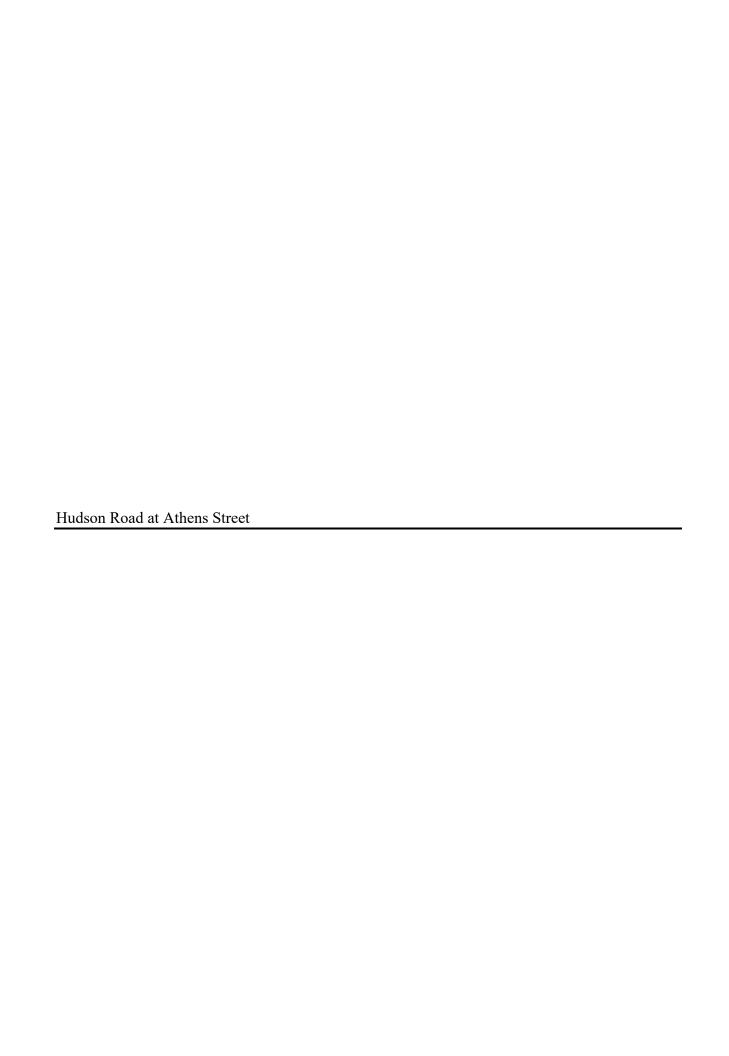
CAPACITY ANALYSIS WORKSHEETS

Route 117 at Hudson Road Hudson Road at Athens Street Hudson Road at Edson Street



Intersection						
	34.1					
		EDD	MA	WOT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4	00	400	र्स	¥	004
Traffic Vol, veh/h	500	83	120	262	79	284
Future Vol, veh/h	500	83	120	262	79	284
Conflicting Peds, #/hr	0	0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	- 4 0	-	-	-	0	-
Veh in Median Storage, 7		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	90	90	92	92
Heavy Vehicles, %	6	5	2	8	5	2
Mvmt Flow	575	95	133	291	86	309
Major/Minor Ma	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	670	0	1180	623
Stage 1	-	-	-	-	623	-
Stage 2	_	_	_	-	557	_
Critical Hdwy	_	_	4.12	_	6.45	6.22
Critical Hdwy Stg 1	_	_		_	5.45	-
Critical Hdwy Stg 2	_	_	_	_	5.45	_
Follow-up Hdwy	_	_	2.218	_	3.545	3 318
Pot Cap-1 Maneuver	_	_	920	_	207	486
Stage 1	_	_	-	_	529	-
Stage 2	_	_	_	_	568	_
Platoon blocked, %	_	_		_	000	
Mov Cap-1 Maneuver	_	_	920	_	171	486
Mov Cap-2 Maneuver	_	_	-	_	171	-
Stage 1					529	_
Stage 2	-	-	-	_	470	_
Stage 2		_	-	_	470	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		3		125.3	
HCM LOS					F	
				EBR	WBL	WBT
Minor Lane/Major Mymt	N	JRI n1	FRI			1011
Minor Lane/Major Mvmt	1	VBLn1	EBT			
Capacity (veh/h)	١	347	-	-	920	-
Capacity (veh/h) HCM Lane V/C Ratio	N	347 1.137	-	-	920 0.145	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	N	347 1.137 125.3	- - -	- - -	920 0.145 9.6	- - 0
Capacity (veh/h) HCM Lane V/C Ratio	N	347 1.137	-	- - -	920 0.145	-

Intersection								
Int Delay, s/veh	112.4							
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
ane Configurations	ĵ.			4	¥			
raffic Vol, veh/h	334	103	359	571	80	208		
uture Vol, veh/h	334	103	359	571	80	208		
onflicting Peds, #/hr	0	0	0	0	0	0		
ign Control	Free	Free	Free	Free	Stop	Stop		
T Channelized	-	None	-	None	-			
orage Length	_	-	_	-	0	-		
eh in Median Storage	e, # 0	_	_	0	0	_		
rade, %	0	_	_	0	0	_		
eak Hour Factor	94	94	97	97	90	90		
eavy Vehicles, %	1	2	1	1	0	0		
wmt Flow	355	110	370	589	89	231		
/Mt Flow	ააა	110	370	209	69	231		
jor/Minor N	Major1	ı	Major2	ı	Minor1			
	0	0	465		1739	410		
onflicting Flow All								
Stage 1	-	-	-	-	410	-		
Stage 2	-	-	-	-	1329	-		
tical Hdwy	-	-	4.11	-	6.4	6.2		
tical Hdwy Stg 1	-	-	-	-	5.4	-		
tical Hdwy Stg 2	-	-	-	-	5.4	-		
llow-up Hdwy	-	-	2.209	-	3.5	3.3		
ot Cap-1 Maneuver	-	-	1102	-	97	646		
Stage 1	-	-	-	-	674	-		
Stage 2	-	-	-	-	250	-		
atoon blocked, %	-	-		-				
ov Cap-1 Maneuver	-	-	1102	-	~ 49	646		
ov Cap-2 Maneuver	-	-	-	-	~ 49	-		
Stage 1	-	-	-	-	674	-		
Stage 2	-	-	-	-	125	-		
_								
proach	EB		WB		NB			
CM Control Delay, s	0		3.8	\$	601.1			
CM LOS				Ψ	F			
inor Lane/Major Mvm	nt 1	NBLn1	EBT	EBR	WBL	WBT		
apacity (veh/h)		147			1102	-		
CM Lane V/C Ratio		2.177	_		0.336	_		
CM Control Delay (s)	¢	601.1			9.9	0		
CM Lane LOS	Ф	F	-					
	١		-	-	A	Α		
CM 95th %tile Q(veh))	26.2	-	-	1.5	-		
tes								
Volume exceeds cap	pacity	\$: De	elay exc	ceeds 3	00s	+: Com	putation Not Defined	*: All major volume in platoon



Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	THE STATE OF THE S	EDR	INDL			SDR
Traffic Vol, veh/h		15	5	લ 286	♣ 151	21
•	60	15	5		151	
Future Vol, veh/h			5	286		21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	95	86	92
Heavy Vehicles, %	2	2	2	3	7	2
Mvmt Flow	65	16	5	301	176	23
Major/Minor	Minor2		Major1		/loior2	
			Major1		Major2	
Conflicting Flow All	499	188	199	0	-	0
Stage 1	188	-	-	-	-	-
Stage 2	311	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	531	854	1373	-	-	-
Stage 1	844	-	-	-	-	-
Stage 2	743	-	-	-	_	-
Platoon blocked, %				-	_	_
Mov Cap-1 Maneuver	529	854	1373	_	_	_
Mov Cap-2 Maneuver	529	-	-	_	_	_
Stage 1	841	_			_	
_	743	_	_	_	_	_
Stage 2	743	_		-	-	_
Approach	EB		NB		SB	
HCM Control Delay, s	12.3		0.1		0	
HCM LOS	В		.			
110111 200						
	nt	NBL	NBT	EBLn1	SBT	SBR
Minor Lane/Major Mvm		4070	_	573	-	-
Capacity (veh/h)		1373				
		0.004		0.142	-	-
Capacity (veh/h) HCM Lane V/C Ratio				0.142 12.3	-	-
Capacity (veh/h))	0.004	-			
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.004 7.6	- 0	12.3	-	-

Intersection						
Int Delay, s/veh	1.2					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	Ą.	
Traffic Vol, veh/h	40	10	17	239	370	69
Future Vol, veh/h	40	10	17	239	370	69
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	_	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	77	83	92
Heavy Vehicles, %	2	2	2	1	0	2
Mymt Flow	43	11	18	310	446	75
IVIVIII(I IOW	40		10	310	טדד	10
Major/Minor	Minor2	- 1	Major1	N	//ajor2	
Conflicting Flow All	830	484	521	0	-	0
Stage 1	484	_	-	-	-	-
Stage 2	346	-	-	-	_	-
Critical Hdwy	6.42	6.22	4.12	_	_	_
Critical Hdwy Stg 1	5.42	-	- 1	_	_	_
Critical Hdwy Stg 2	5.42	_			_	_
		3.318	2 210	_		_
Follow-up Hdwy				-	-	
Pot Cap-1 Maneuver	340	583	1045	-	-	-
Stage 1	620	-	-	-	-	-
Stage 2	716	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	333	583	1045	-	-	-
Mov Cap-2 Maneuver	333	-	-	-	-	-
Stage 1	607	-	-	-	-	-
Stage 2	716	-	-	-	-	-
J -						
Approach	EB		NB		SB	
HCM Control Delay, s	16.6		0.5		0	
HCM LOS	С					
Minor Long (Maior M		NDI	NDT	CDL 4	CDT	CDD
Minor Lane/Major Mvn	11	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1045	-		-	-
HCM Lane V/C Ratio		0.018	-	0.149	-	-
HCM Control Delay (s)		8.5	0	16.6	-	-
HCM Lane LOS		Α	Α	С	-	-
HCM 95th %tile Q(veh)	0.1	-	0.5	-	-



Intersection						
Int Delay, s/veh	2.9					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N/		4	_		ની
Traffic Vol, veh/h	17	67	224	7	36	130
Future Vol, veh/h	17	67	224	7	36	130
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	_	0	-	-	0
Peak Hour Factor	71	71	97	97	86	86
Heavy Vehicles, %	0	6	3	0	7	7
Mvmt Flow	24	94	231	7	42	151
IVIVIII (I IOW	27	J-1	201	•	72	101
Major/Minor	Minor1		//ajor1	I	Major2	
Conflicting Flow All	470	235	0	0	238	0
Stage 1	235	-	-	-	-	-
Stage 2	235	_	-	_	_	_
Critical Hdwy	6.4	6.26	-	-	4.17	-
Critical Hdwy Stg 1	5.4	-	_	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.354	_	_	2.263	_
Pot Cap-1 Maneuver	556	794		_	1300	_
	809	1 34	-	-	1300	_
Stage 1			-	_	_	-
Stage 2	809	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	537	794	-	-	1300	-
Mov Cap-2 Maneuver	537	-	-	-	-	-
Stage 1	809	-	-	-	-	-
Stage 2	781	-	-	-	-	-
Annragah	MD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	10.9		0		1.7	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NRRV	VBLn1	SBL	SBT
	II.	וטוו	ואוטויו			
Capacity (veh/h)		-	-		1300	-
HCM Lane V/C Ratio		-	-	0.163		-
HCM Control Delay (s)		-	-	10.9	7.9	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh		-	-	0.6	0.1	-

Intersection						
Int Delay, s/veh	2.5					
Movement	WBL	WBR	NDT	NBR	SBL	SBT
		WDK	NBT	NDK	ODL	
Lane Configurations	Y	74	}	10	71	વ
Traffic Vol, veh/h	14	71	185	19	74	306
Future Vol, veh/h	14	71	185	19	74	306
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	71	71	78	78	83	83
Heavy Vehicles, %	0	6	0	0	0	0
Mvmt Flow	20	100	237	24	89	369
			_0,			
Major/Minor	Minor1		//ajor1	N	Major2	
Conflicting Flow All	796	249	0	0	261	0
Stage 1	249	-	-	-	-	-
Stage 2	547	-	-	-	-	-
Critical Hdwy	6.4	6.26	_	_	4.1	-
Critical Hdwy Stg 1	5.4	_	-	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy		3.354	_	_	2.2	_
Pot Cap-1 Maneuver	359	780	_	_	1315	_
Stage 1	797	700	_	_	1313	_
				-	_	
Stage 2	584	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	328	780	-	-	1315	-
Mov Cap-2 Maneuver	328	-	-	-	-	-
Stage 1	797	-	-	-	-	-
Stage 2	534	-	-	-	-	-
Annach	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	12		0		1.5	
HCM LOS	В					
Minor Lane/Major Mvn	nt	NBT	NRRV	VBLn1	SBL	SBT
		-	-		1315	- 100
Capacity (veh/h)				636		-
HCM Cartral Dalay (a)		-	-	0.188		-
HCM Control Delay (s)		-	-	12	7.9	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)	-	-	0.7	0.2	-